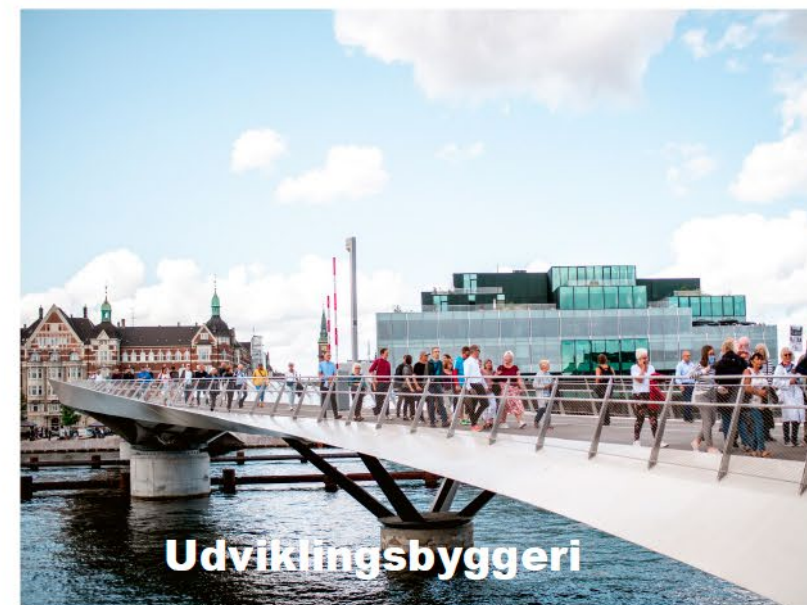


MINICO 2 ETAGEHUS BETON



VI SKABER LIVSKVALITET I DET BYGGEDE MILJØ GENNEM EJERSKAB

Udvikler og bevarer



TEAM

Udvikler og bevarer



BYGHERRE



INGENIØRER



ARKITEKTER

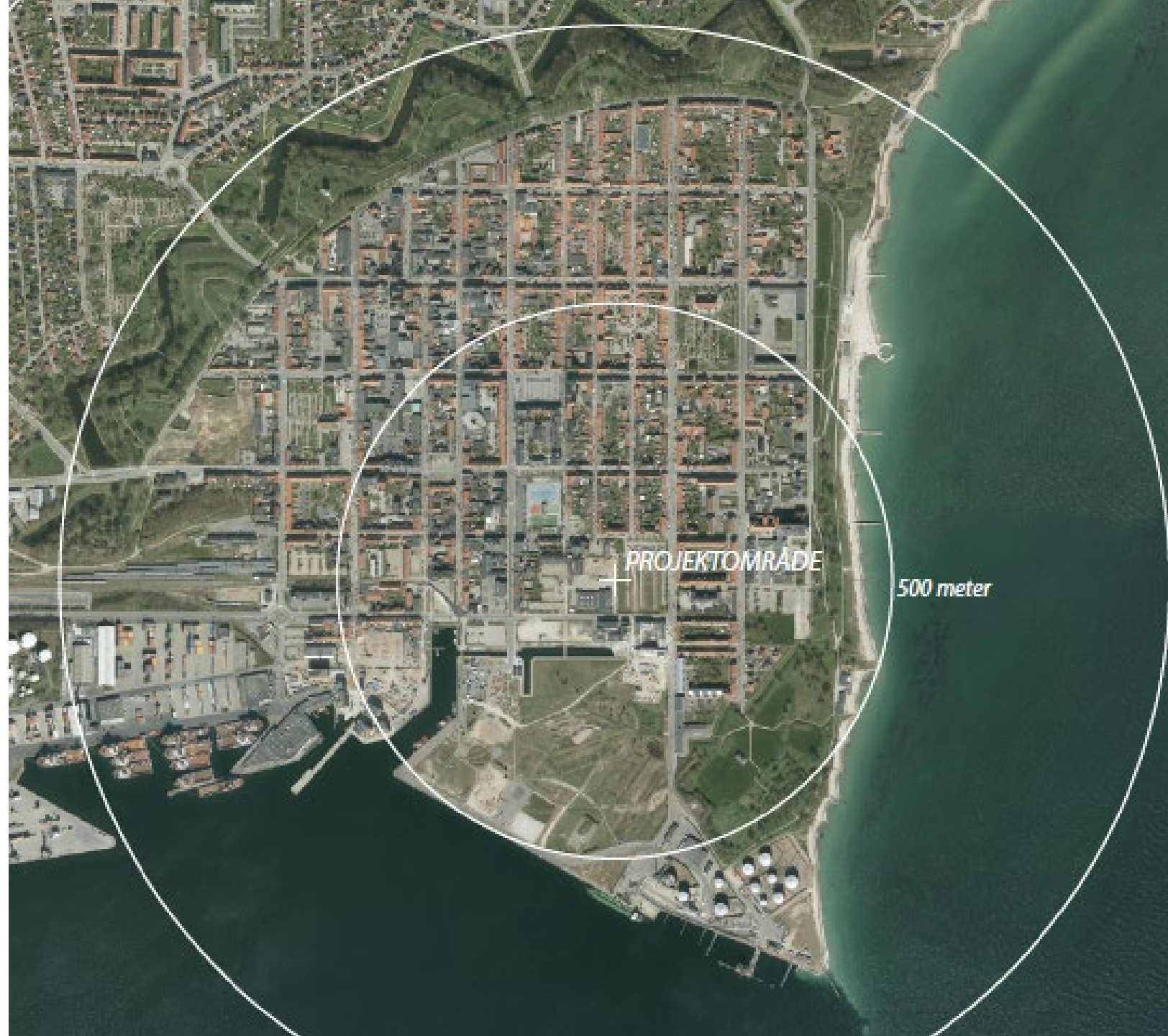


HOVEDENTREPRENØR



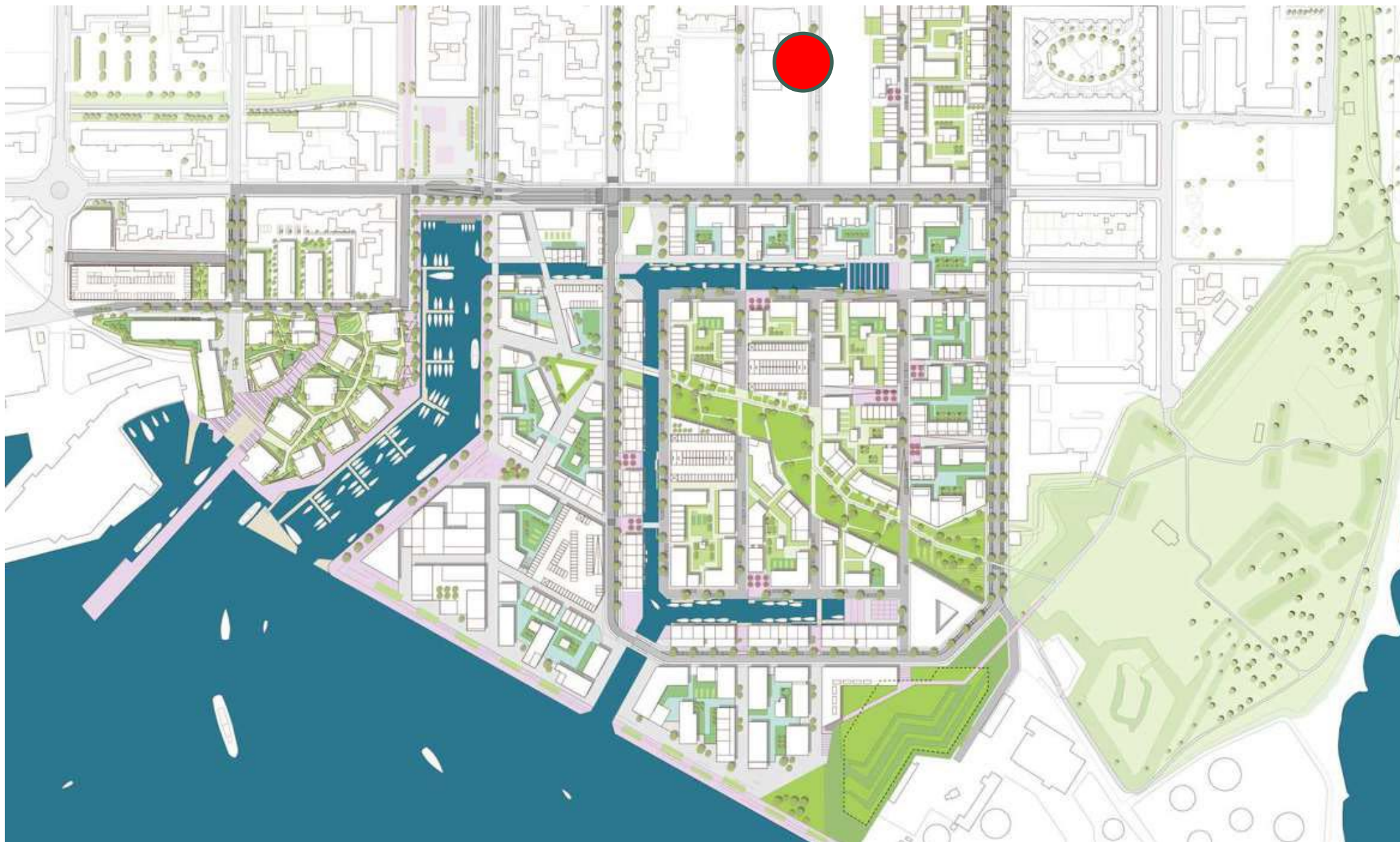
STED

KONGENSSTRÆDE 88
KANALBYEN FREDERICIA



KANALBYEN, FREDERICIA

Udvikler og bevarer

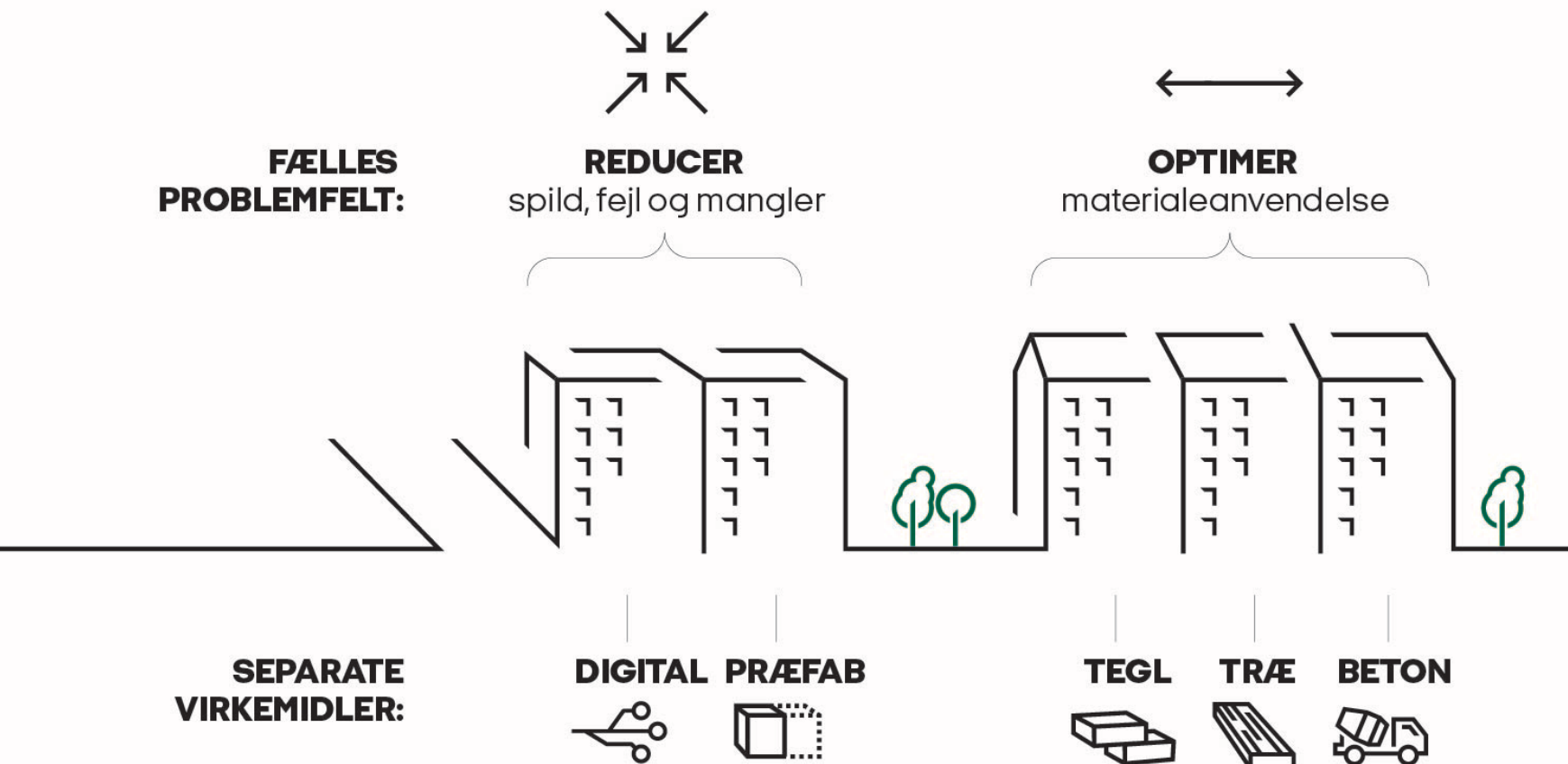


MINICO2 ETAGEHUSENE – ET KOMPARATIVT STUDIE

Udvikler og bevarer



Realdania
By & Byg



- 5 ETAGEHUSE
- 1 GADESIDE
- 5 ETAGER
- 600 M2
- SAMME VOLUMEN
- SAMME BOLIGPLAN

MAX. BETON
GNSN. PRIS
MIN. CO₂

FORSØGSDESIGN – 25 SPØRGSMÅL

Udvikler og bevarer



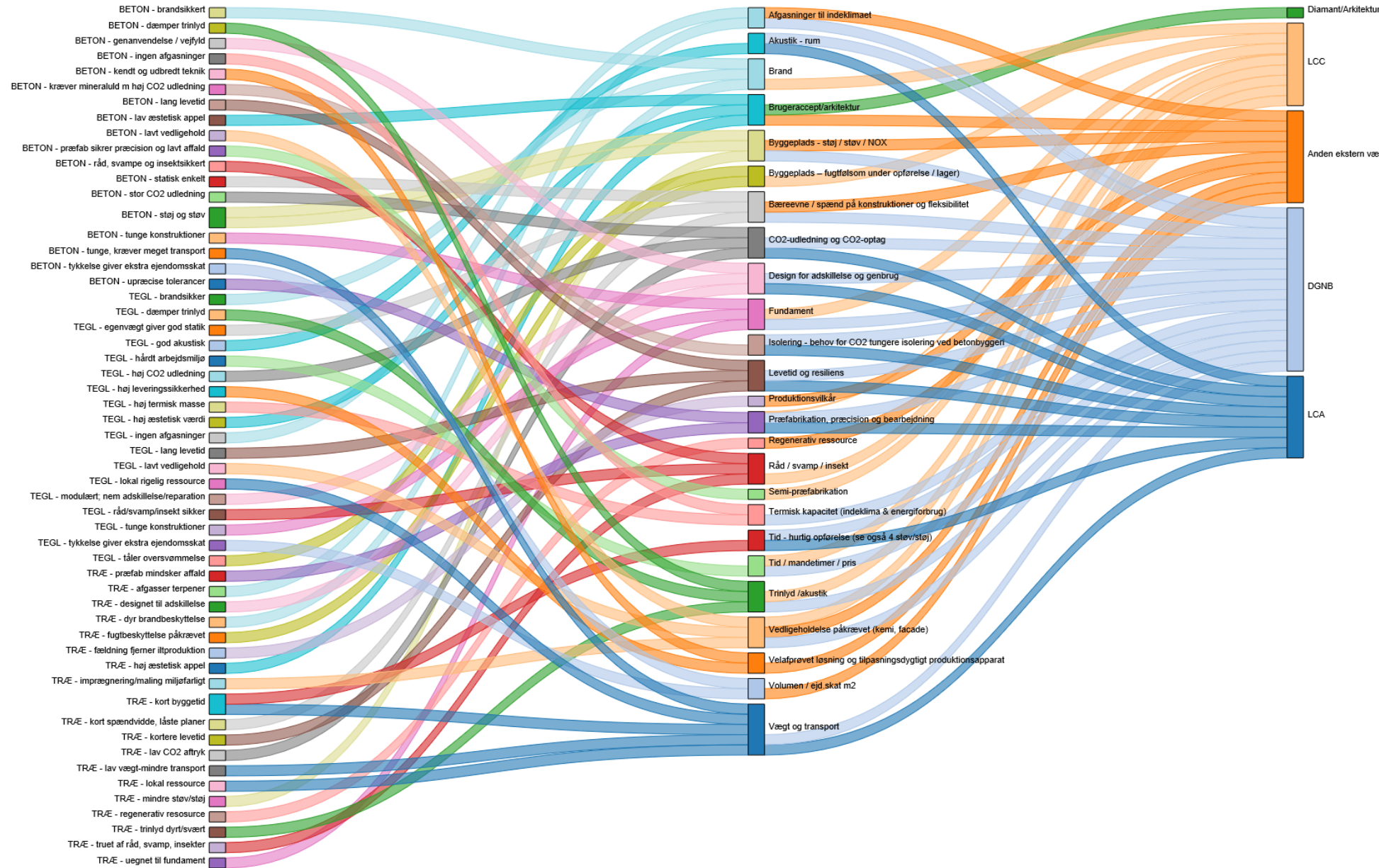
ARKITEKTUR

PRIS

BRUGER

DGNB

CO₂



BRAND

UORGANISK

PRODUKTION

ELEMENTER
IN-SITU

STATIK

KK3
ROBUSTHEDSKRAV
BEREGNINGSMETODIK

RESSOURCER

KNAPHED
GENBRUG
BIODIVERSITET

AKUSTIK

BR DK
FREKVENSER

CO2

CEMENT
HÆRDETID
GENBRUG

ÆSTETIK

PERCEPTION
HISTORIE
PATINERING

**TIDLIGERE ERFARING HOS REALDANIA BY&BYG
- GLASALSTRUP, HASSELAGER (1967)**

Realdania

Realdania
By & Byg





**TIDLIGERE ERFARING HOS REALDANIA BY&BYG
- KNUD FRIIS VILLA, BRABRAND (1958)**

Realdania

Realdania
By & Byg





MINICO2 ETAGEHUS BETON/TRÆ/TEGL - KONGENSSTRÆDE 88-92, FREDERICIA

Udvikler og bevarer



DOBBELT BUNDLINJE KONSTRUKTIONER ARKITEKTUR

DOBBELT BUNDLINJE

KONSTRUKTIONER

ARKITEKTUR

0.25

03 | BYGNINGSDELE

05 | DÆK

5.1 | ETAGEDÆK
5.1.4 | INSITU DÆK
C16 UNIGREEN PLUS**
120MM

INSITU DÆK C16 UNIGREEN PLUS** 120MM.
Standard in-situ betondek med lav betonsyrke og optimeret armeringsmængde.

Fordele
Meget Lav CO2 belastning
Høj robusthed og fleksibilitet
Tyndt etagedæk uden bjælker
Lidt lavere pris

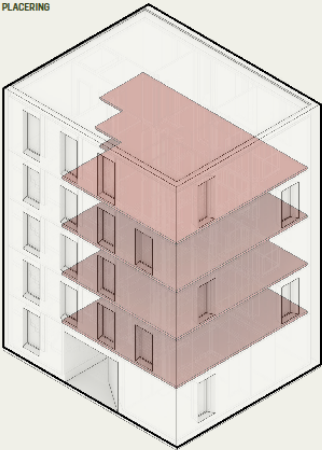
Ulemper
Længere afbørningstid

[**]Produktet er på markedet, men GWP tal oplyst af producent, som afhænger af projektspecifik EPD.



Insitu dæk C16 Unigreen plus**

PLACERING



PRISKLASSE

400 - 600

Kr./m²

LCA | GWP



0.24

03 | BYGNINGSDELE

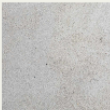
05 | DÆK

5.2 | GULV
5.2.1 | BETONGULV
25MM GLITTET

BETONGULV 25MM GLITTET
Udføres med 5 mm tykkelse af genanvendt gummi fra bildekk og et meget tyndt afretningslag i beton med rensarmring. Glattet overflade.

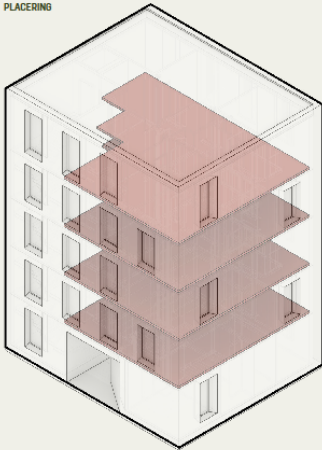
Fordele
Optimeret betongulv

Ulemper
Risiko for rener
Opmærksomhed om lydransmissioner.
Høj krav til udløbss



Betongulv

PLACERING

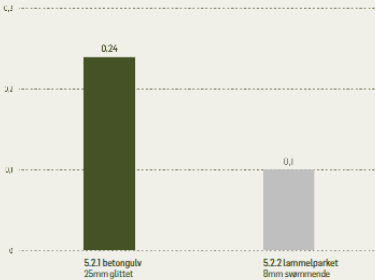


PRISKLASSE

700 - 1050

Kr./m²

LCA | GWP



0.60

03 | BYGNINGSDELE

03 | YDERVÆGGE

3.0.3 | MURET GENBRUGSFACADE OG C20 ELEMENT*** 80MM

MURET GENBRUGSFACADE OG C20 ELEMENT* 80MM**
Optimeret betonelement facade med 80 mm bagvæg, 300 mm blød isolering og 80 mm forplade i genbrugsbeton.

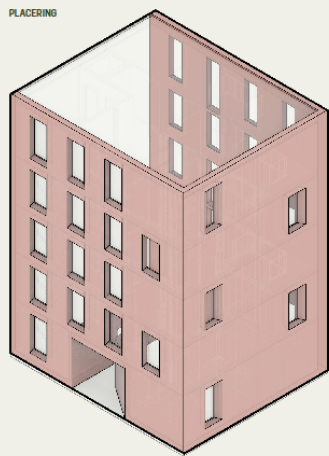
Fordelte
Optimeret
Minimalt Resourcenforbrug
Gendrag af betonelementer
Mulighed for bløgen isolering

Ulemper
Kort hærdetid på grund af løft af elementer
Tidskrævende opmuring af formur
Pris på genbrugte betonen

*** Fremskrevet GWP af producent, som vurderes markedsparat indenfor 6 mdr.



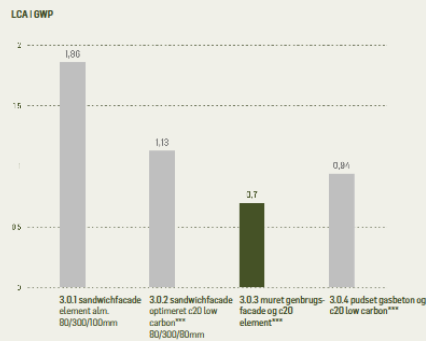
Muret genbrugsfacade og C20 element***



PRISKLASSE

6500 - 13000

Kr./m²



0.80

03 | BYGNINGSDELE

03 | YDERVÆGGE

3.0.4 | PUDSET GASBETON OG C20 ELEMENT LOW CARBON*** 80MM

PUDSET GASBETON OG C20 ELEMENT LOW CARBON* 80MM**
Optimeret betonelement facade med 80 mm bagvæg, 300 mm blød isolering og 80 mm forplade i porebetonblokke.

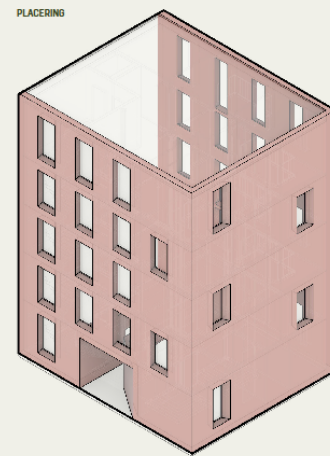
Fordelte
Optimeret
Mulighed for bløgen isolering
Lav pris

Ulemper
Kort hærdetid på grund af løft af elementer
Tidskrævende opmuring af formur

*** Fremskrevet GWP af producent, som vurderes markedsparat indenfor 6 mdr.



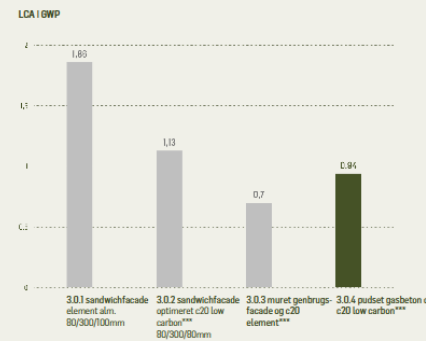
Pudset gasbeton og C20 element low carbon***



PRISKLASSE

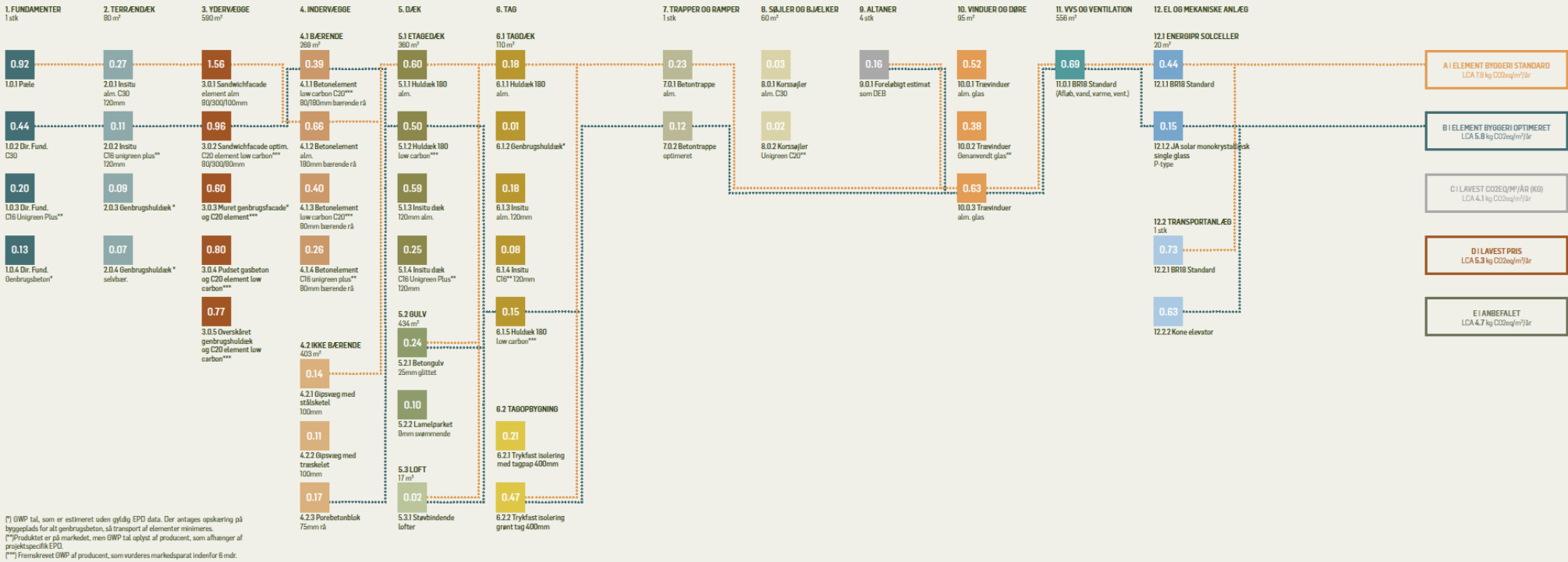
1500 - 4000

Kr./m²



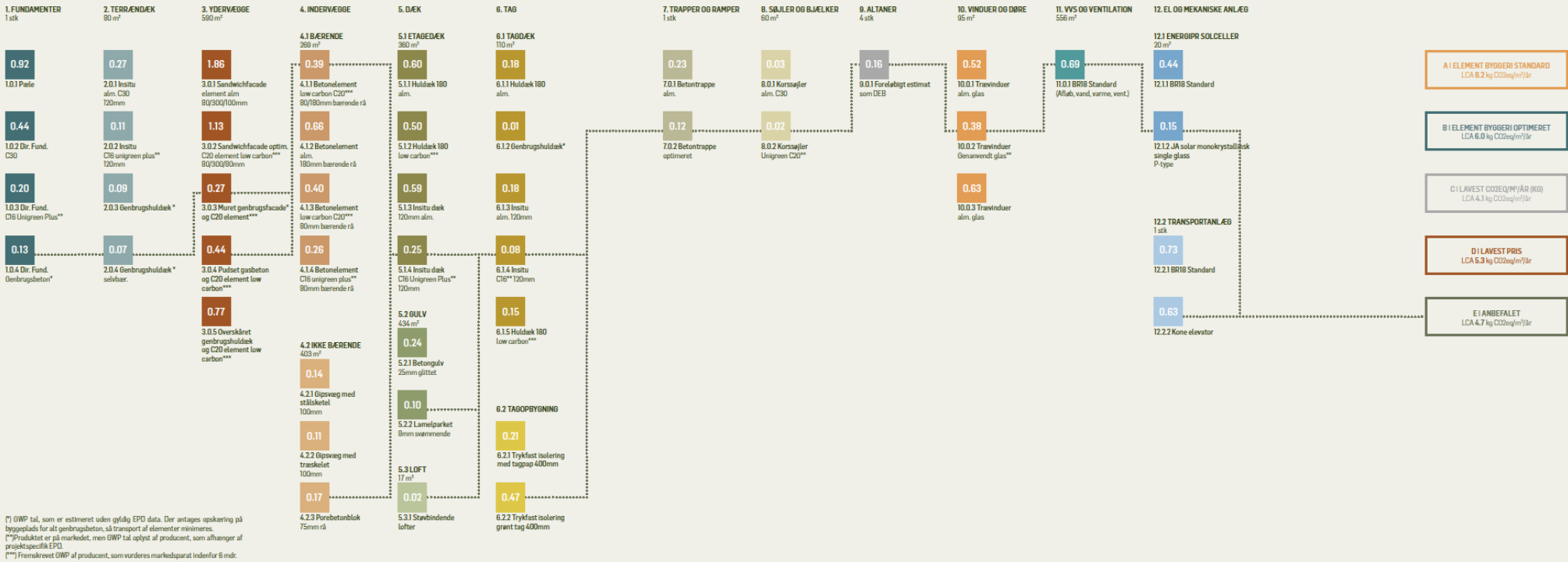
01 | OVERSIGT

A | ELEMENTBYGGERI STANDARD
B | ELEMENTBYGGERI OPTIMERET
C | LAVEST CO2EQ/M²/ÅR (KG)
D | LAVEST PRIS
E | ANBEFALET

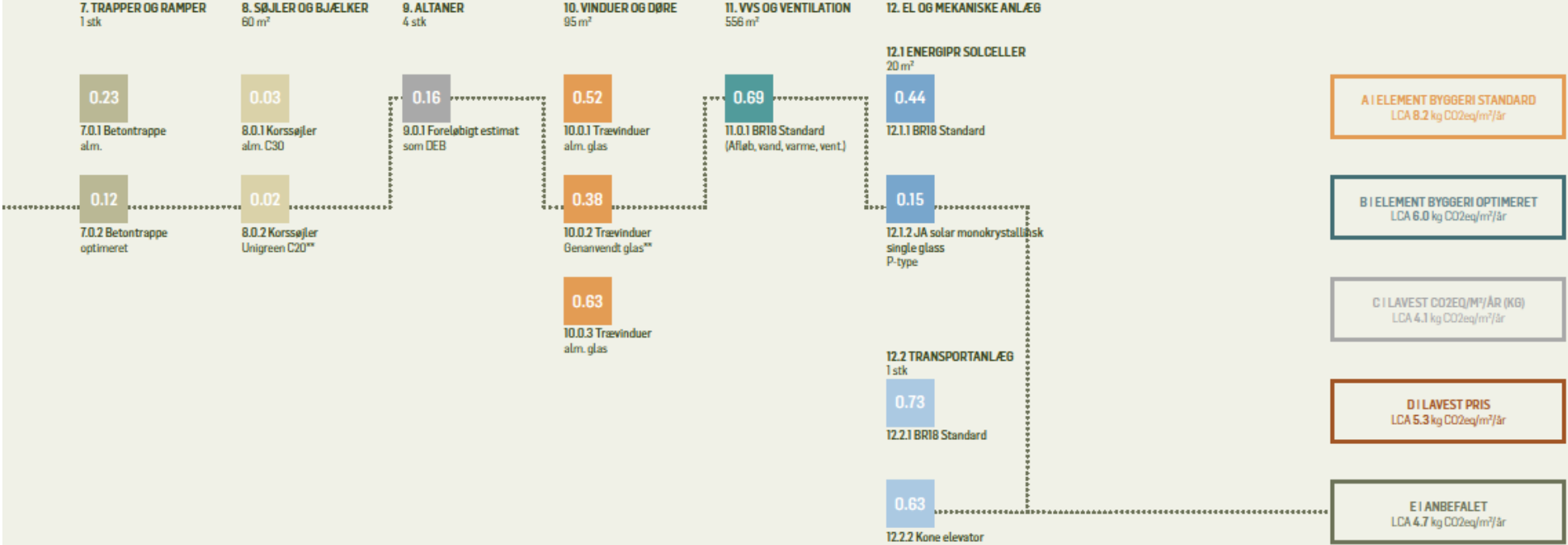


01 | OVERSIGT

A | ELEMENTBYGGERI STANDARD
B | ELEMENTBYGGERI OPTIMERET
C | LAVEST CO2EQ/M²/ÅR (KG)
D | LAVEST PRIS
E | ANBEFALET



DOBBELT BUNDLINJE



02 | ANALYSE

E | ANBEFALET

ANBEFALET

Den anbefalede variant er den, der har det lavest CO₂ med mest muligt beton og en pris på cirka 20.000 kr/m². Udføres med pladstøbte dæk i beton, betonsøjler og bærende betonelement ydervægge. Facader, fundamenter og terrændæk udføres delvist med genbrugte betonelementer.

Fordele

Lav CO₂
Pris indenfor forventet ramme
Mest muligt anvendelse af beton
Høj anvendelse af kendte byggeprincipper
Høj fleksibilitet og robusthed
Anvendelse af genbrug

Ulemper

Lidt længere byggetid
Nye principper for genanvendelse af beton

(*) GWP tal, som er estimeret uden gyldig EPD data. Der antages opskæring på byggeplads for alt genbrugsbeton, så transport af elementer minimeres.
(**) Produktet er på markedet, men GWP tal oplyst af producent, som afhænger af projektspecifik EPD.
(***) Fremskrevet GWP af producent, som vurderes markedsparat indenfor 6 mdr.

9.0.1 Altaner
foreløbigt estimat 0.16

10.0.2 Trævinduer
genanvendt glas** 0.38

3.0.3 Muret genbrugs
facade* og C20 element*** 0.27

3.0.4 Pudset gasbeton
og C20 element low
carbon*** 0.44

8.0.2 Korsøjler
unigreen C20** 0.02

7.0.2 Betontrappe
optimeret 0.12

11.0.1 BR18 Standard
(afløb, vand, varme,
vent.) 0.69

0.15 12.1.2 Ja Solar
monokrystallinsksingle
glas, P-Type

0.63 12.2.2 KONE elevator

0.47 6.2.2 Trykfast isolering
grønt tag 400mm

0.08 6.1.4 Insitu C16**
120mm

0.02 5.3.1 Støvbinder
løfter

0.10 5.2.2 Lammelparket
8mm svømmende

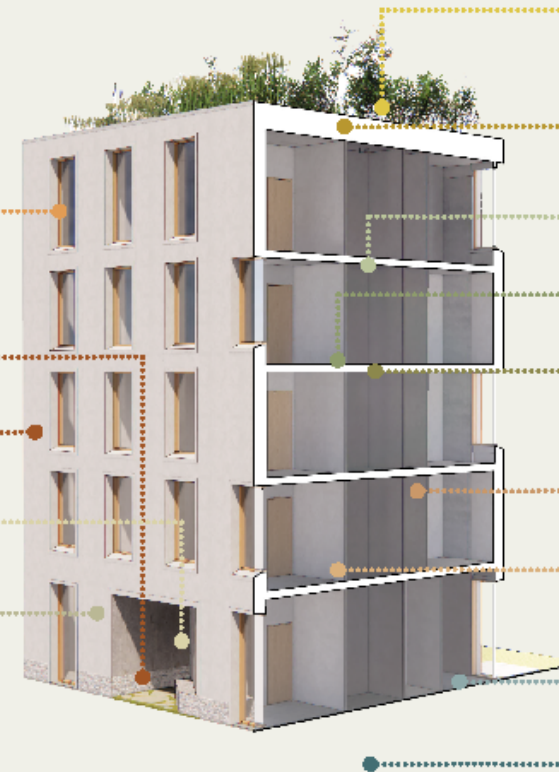
0.25 5.1.4 Insitu dæk
C16 unigreen plus***
120mm

0.39 4.1.1 Betonelement
low carbon C20***
80mm bærende rå

0.17 4.2.3 Porebetonblok
75mm rå

0.07 2.0.4 Genbrugshuldæk*
selvbærende

0.13 1.0.4 Direkte fundering
genbrugsbeton*



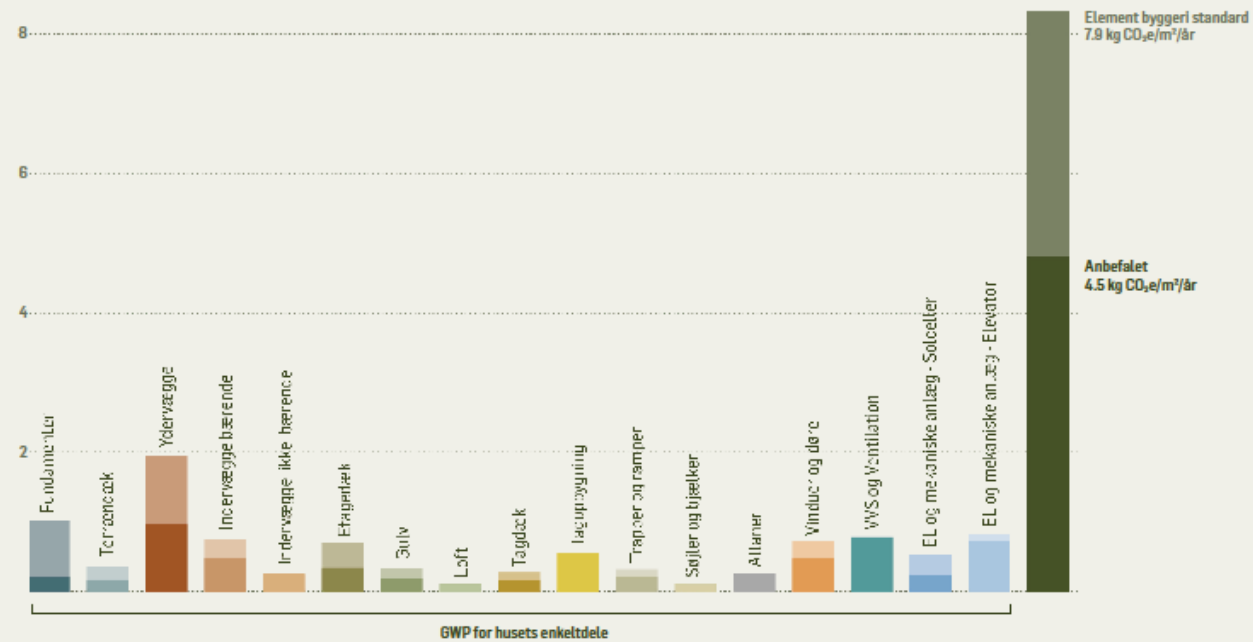
KONKLUSION

556 m²
Bruttoareal

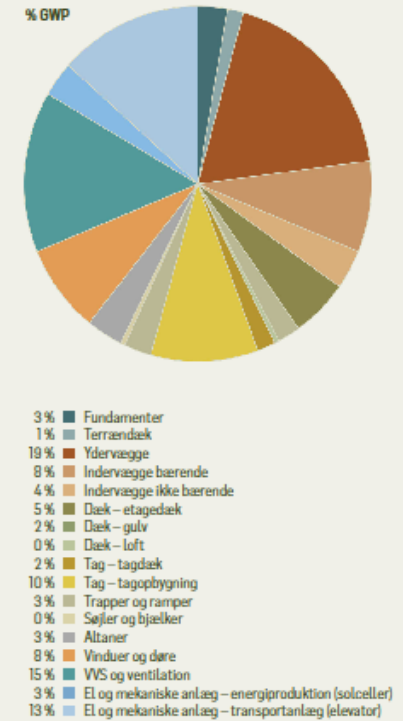
19.000 - 21.000 /kr. m²
Pris

LCA i GWP

Den øjeblikkelige opnåede baseline beregning baseret på forslagets udformning



% GWP



DOBBELT BUNDLINJE **KONSTRUKTIONER** ARKITEKTUR

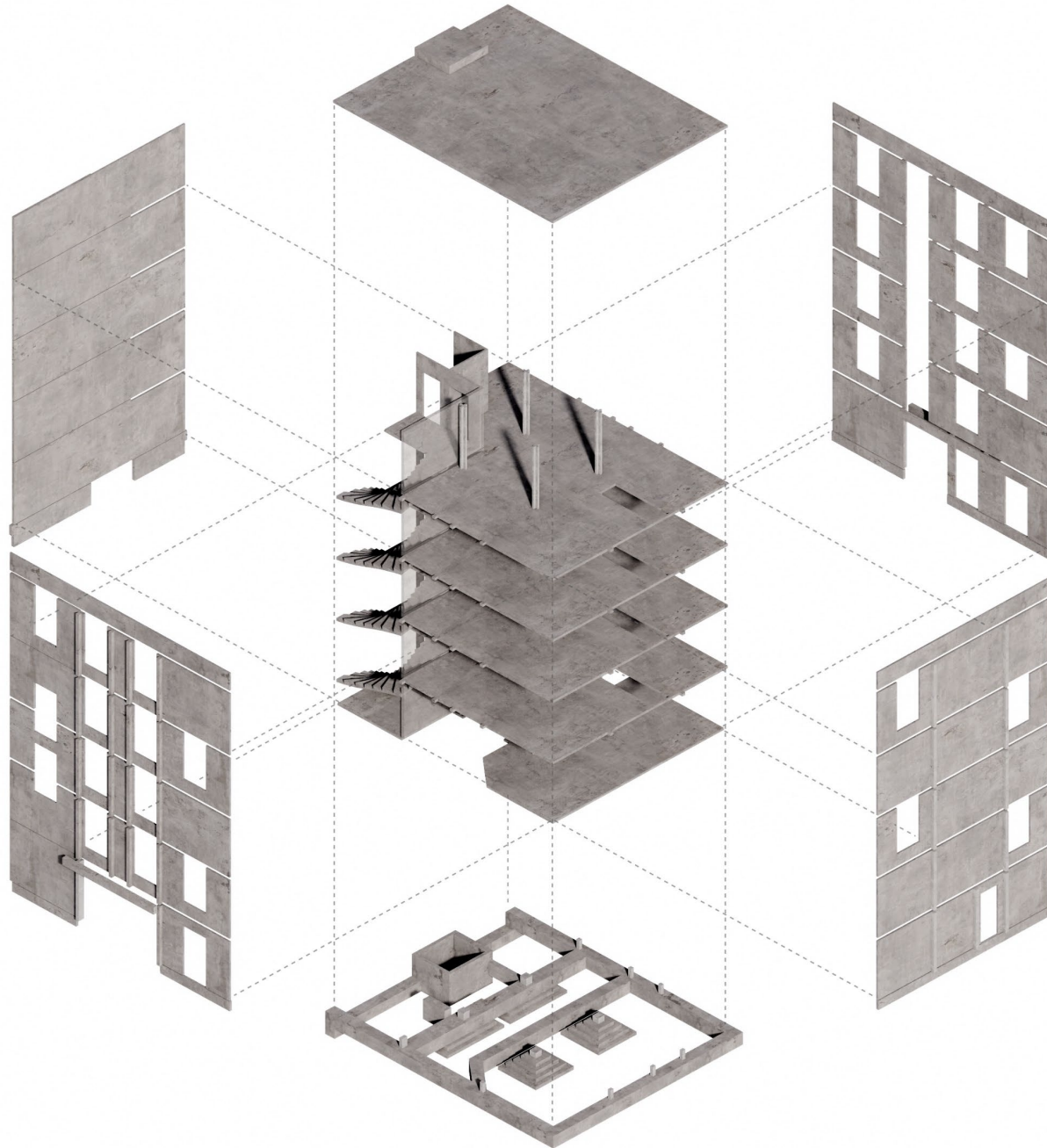
KONSTRUKTIONER

In-situstøbte betondæk
og trappe

Betonelement søjler og
bærende vægge

Fundamenter i in-situ og
genbrugselementer

CO2 reduceret Beton
C16

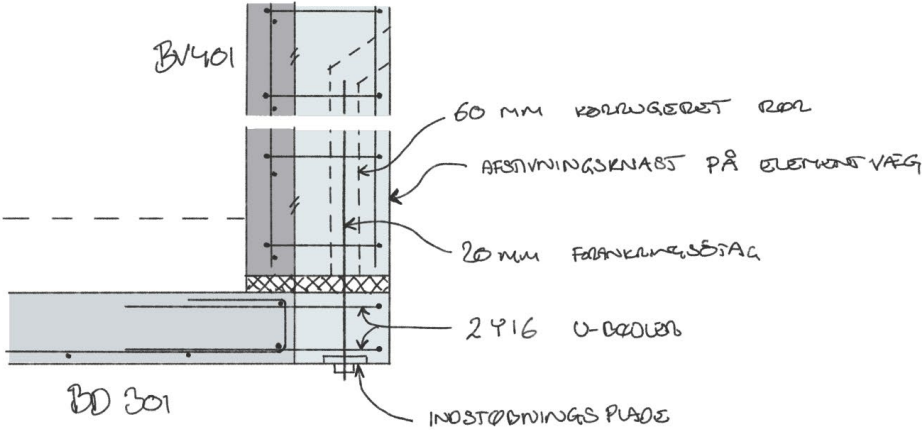
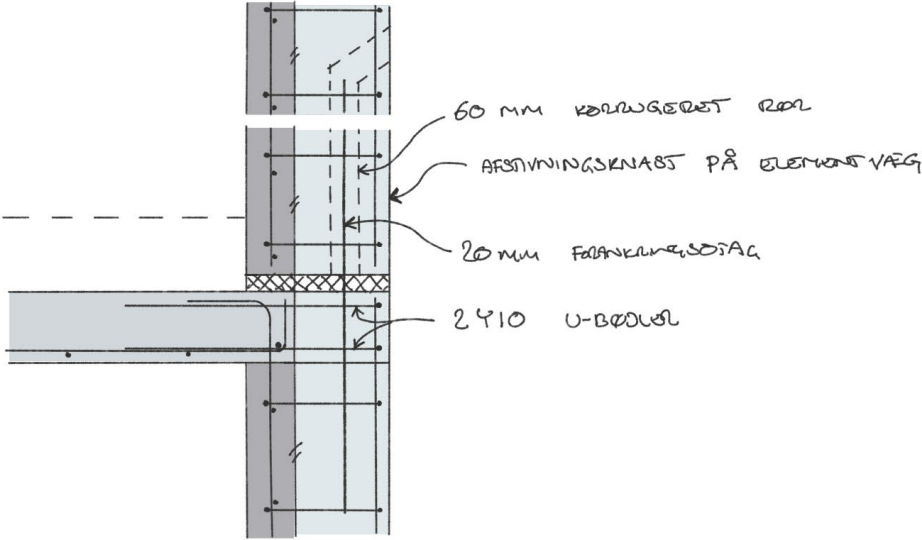
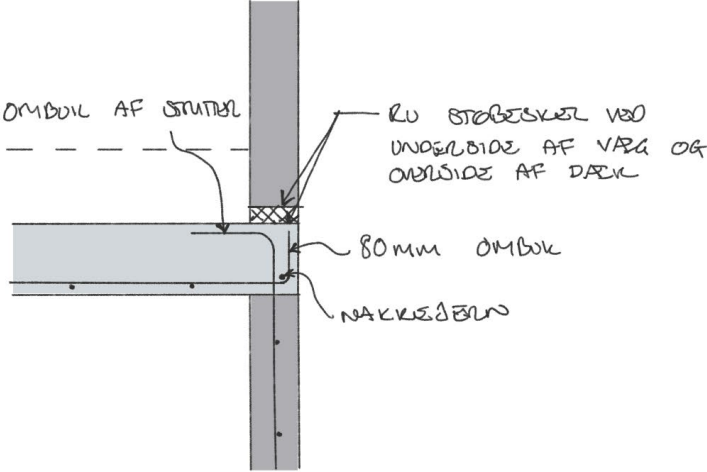


KONSTRUKTIONER

Ydervægge
Lodret snit

12 cm dæk

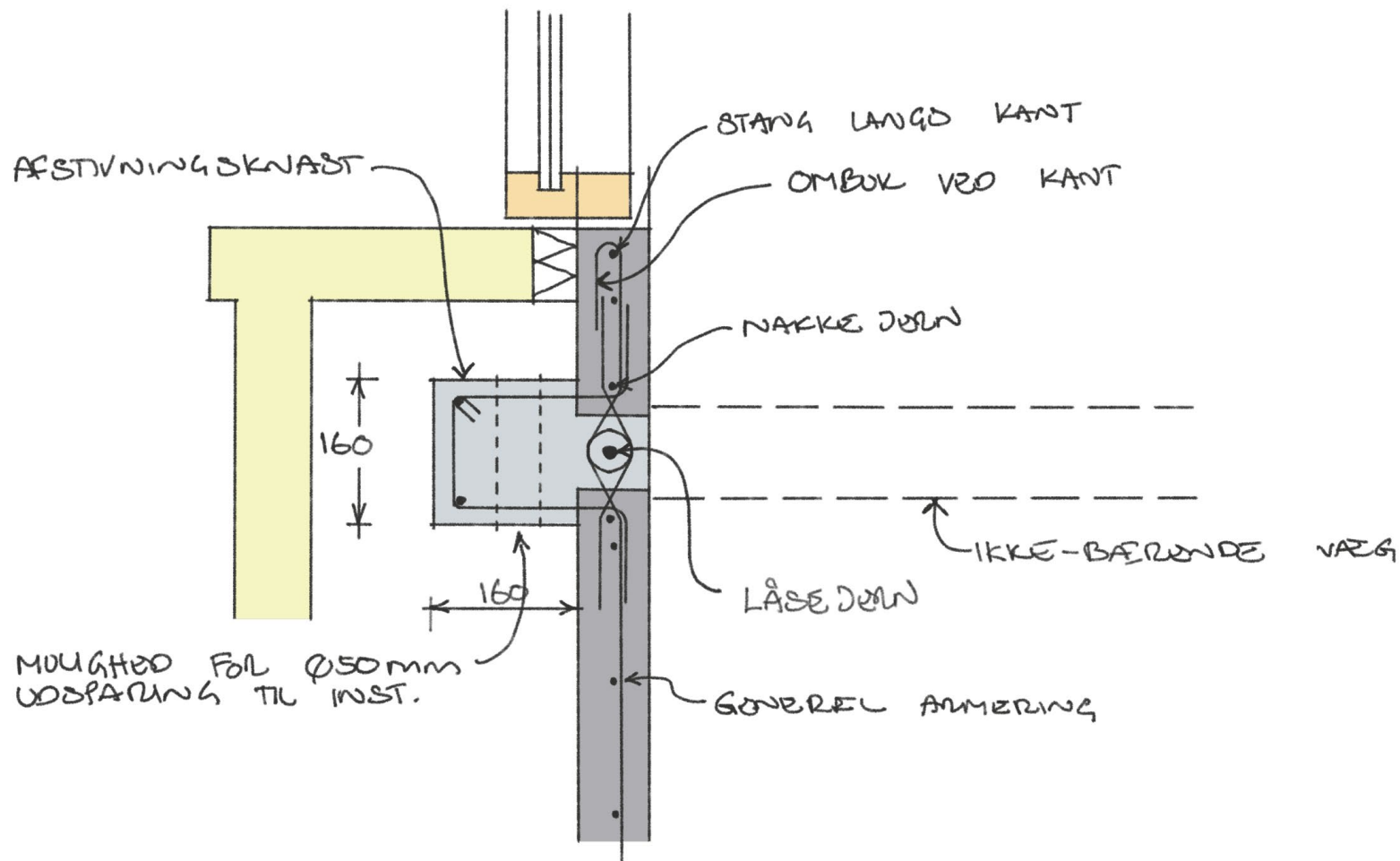
8 cm bærende
betonelement
ydervægge



KONSTRUKTIONER

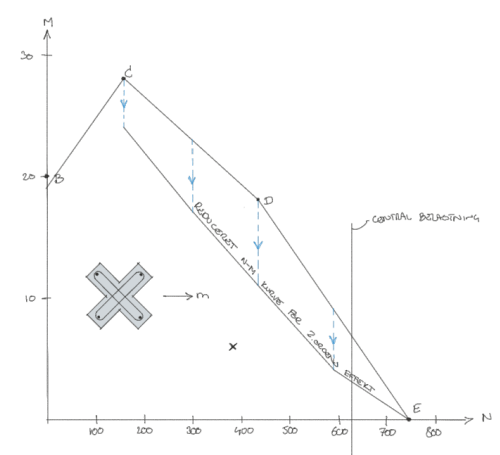
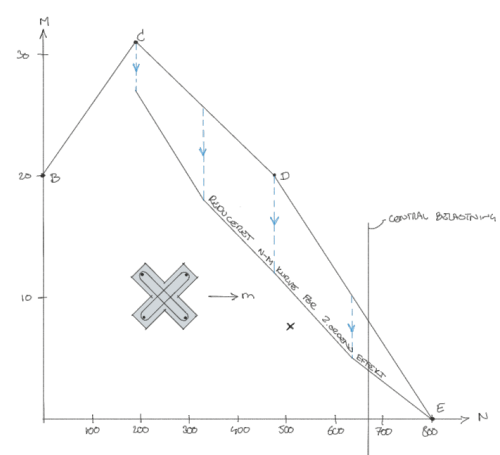
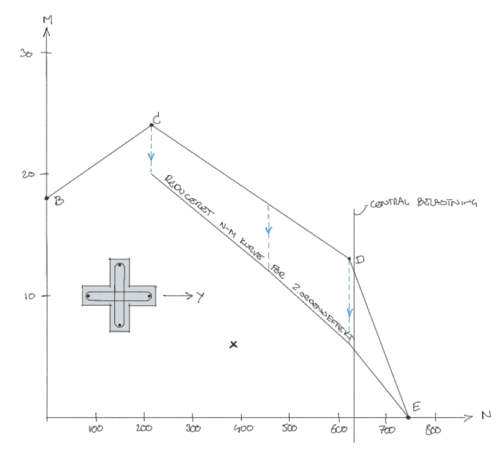
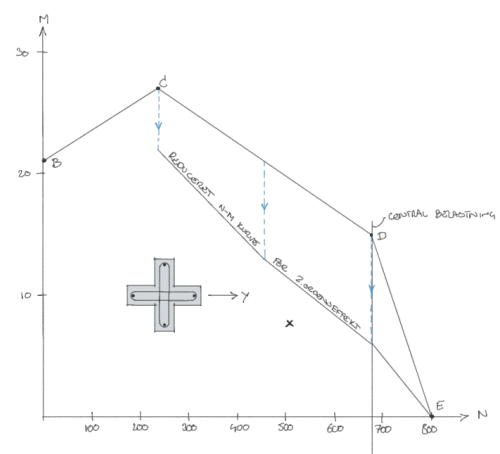
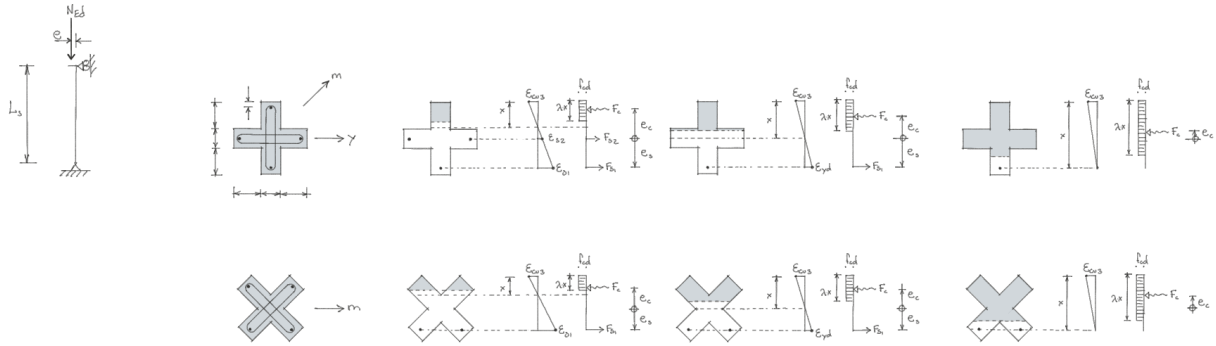
Ydervægge
Vandret snit

8cm betonvæg
med knast



KONSTRUKTIONER

Korssøjler



KONSTRUKTIONER



Inspiration fra klassiske trapper

In-situstøbt trappe Indstøbt i de stabiliserende vægge

evening meeting: price/rogers

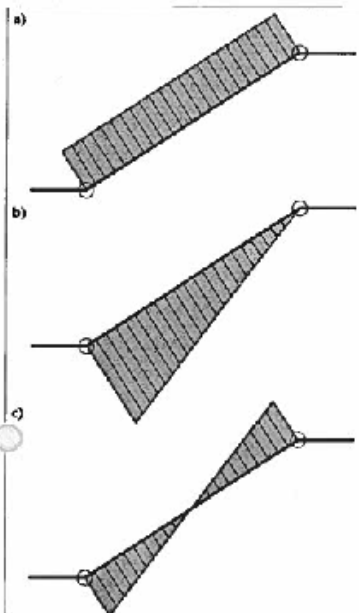


Fig 15 (Left) Diagram showing the torsion in a staircase made with rebated treads under a uniform applied load. (a) Torsion due to restraint forces. (b) Torsion due to load. (c) Resultant torsion (courtesy Ian Russell).

Fig 16. (Below left) Diagram showing the deflected shape of a rebated flight (courtesy Ian Russell). Fig 17. (Below) Landing within a flight of the Beauty Stair at Hampton Court (photo courtesy Sam Peles).

very considerably the stresses in the treads.

A flight with rebated treads has even more unknowns than a flight with plain treads. To simplify the analysis the conservative assumption can be made that the horizontal restraint is only applied at the landings and that the horizontal force is therefore constant throughout the flight, producing a constant couple to all the treads (Fig 15a). In the case of a straight flight with stone landings at the top and bottom, the landings can restrain the flight, and so develop the horizontal force that reduces the torsion in the treads. The couple due to the vertical loads still increases down the flight, so for the plate tread (Fig 15b). The resultant net torsions in the treads are shown in Fig 15c. So, for the flight shown schematically in Fig 16 there is a net clockwise torsion on the lower treads, where the moment from the vertical loads is greater than the moment from the horizontal force, and there is a net anti-clockwise torsion in the upper treads, where the moment from the horizontal force is greater than the moment from the vertical loads. The middle tread is in balance. The rotations are symmetrical, giving the deflected shape shown in Fig 16. The mid-flight deflection will be a quarter of the deflection at the top of a similar flight of unrebated treads.

Fig 15a shows the effect of the rebates. The maximum

torsion on a tread is halved. The stresses in the supporting wall are reduced even more significantly, because the torsion at the wall will be largely provided by the stone-to-stone connection. The wall only has to supply the additional torsion to carry the load from that tread, rather than the whole torsion that would be required by unrebated treads.

It is acknowledged that there are a lot of unknowns in this simplified analysis of a flight made with rebated treads. The value of the horizontal force between the treads is difficult to assess, as it depends not only on the stiffness of the stone and of the supporting wall but also on the end restraints to the flight i.e. how the flight is supported, both vertically and horizontally, at the landings. The detailing of the stone in the flights and landings could affect the way the staircase works, as could the way in which it was built.

One way of testing the accuracy of this analysis is by measuring the deflection in a real flight under a load test. We have had the opportunity to do this on a few occasions, and the actual deflections have always been less than predicted, and usually so small as to be difficult to measure. This indicates that the assumptions made in this simple analysis are conservative.

Modern scientific analysis may not be able to predict accurately the stresses in the treads, but the builders of the late 18th century knew what was going on. They put their faith in the rebates and cut the treads down to the elegant triangular section that became the standard for the 18th century. This reduced the torsional strength of the tread to about a third of its rectangular counterpart and reduced the stiffness to about a sixth. Such dramatic reductions could only be possible with rebated treads.

Landings within the flights and at the top of the flight

The Fountain Court at Hampton Court contains a wonderful collection of stone staircases designed by Wren, or more probably Talman. The 'Beauty Staircase' has a landing within the flight (Fig 17). Here the landing slabs are rebated onto each other, and the load is transmitted in the same way as it

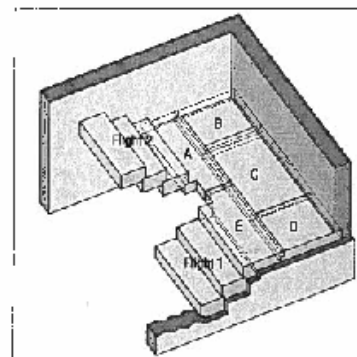


is between the treads in the flight

Similar detailing is normally used at landings at the top of staircases, which can be made up of many separate pieces rebated to each other. An alternative to the rebated joint is the jagged joint, normally lead-filled.

Landings at changes of direction

Again, at Hampton Court, it is possible to see how the flights and landings are interacting by looking at the detailing of the rebates. The general arrangement, for quarter or half landings, is to build the corner pieces of the landings into the two walls of the stairwell. As these pieces of stone are supported



on two walls at right angles they can provide a rigid support for the flight or landing.

This means that in effect the whole staircase is made up of structurally independent flights, with pieces of the landing sitting between. In Fig 18 the stones making up the half landing are lettered A to E. The lower Flight 1 is propped horizontally by stones C, D and E, with stone E spanning between the top of the flight and stone D, which is built into both walls. The load of the upper Flight 2 is carried by landing slab A on the corner slab B, which is built into both walls. In some of the stairs a crack has opened along the line of the rebate between the top tread and the landing, i.e. between stone E, which shows that the horizontal force from the landing has not held the top of the flight (probably due to the rebate being too shallow). It also shows that the lower Flight 1 is not supporting the landing.

Spiral staircases

Spiral stairs with unrebated treads behave in principle in the same way as straight flights of unrebated treads. The



Fig 18. (Left) Diagram showing the rebates between stones in a typical half landing (courtesy Ian Russell).

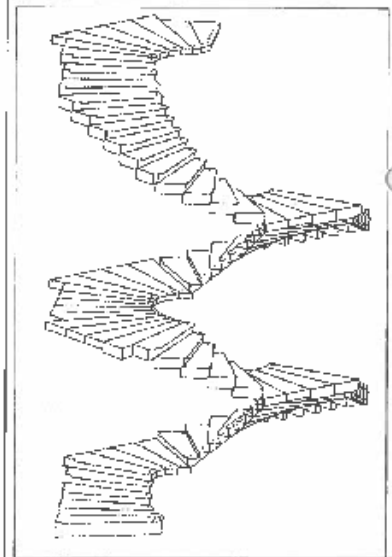
Fig 19. (Below left) Broken tread (photo courtesy Russell Taylor).

Fig 20. (Right) 3D model of new staircase at Corbett Hall (courtesy Helen Rogers). Fig 21. (Below) Construction of a new staircase.

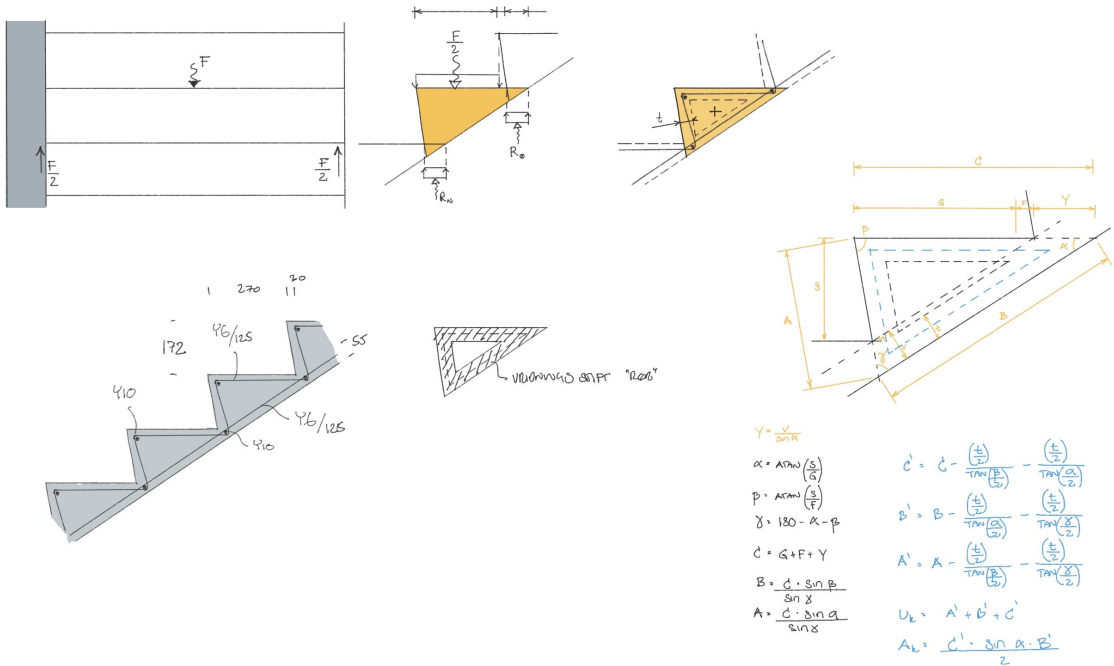
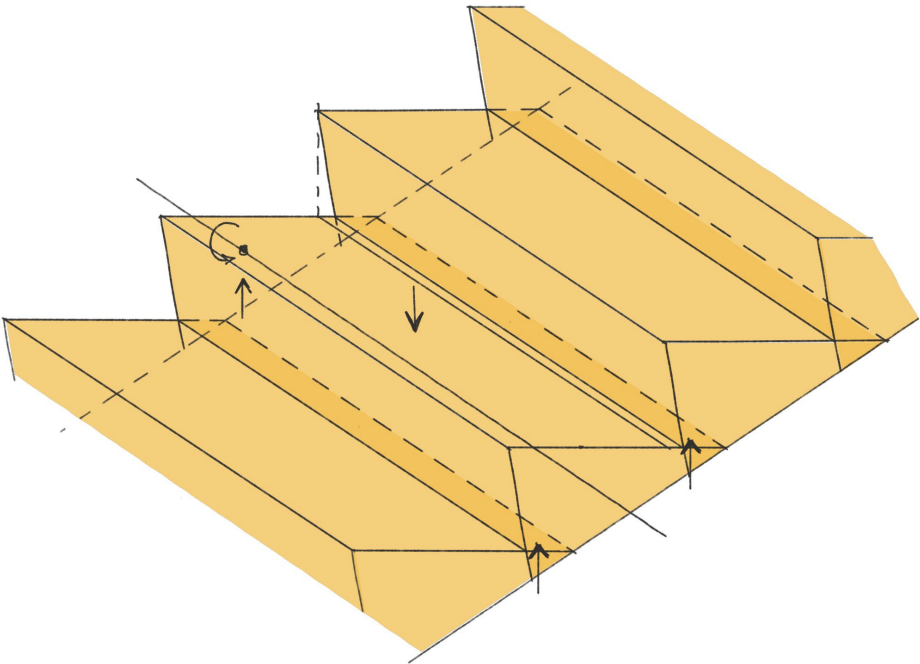
torsional stresses in the treads are less than the stresses in a straight flight because the width of the tread at the outer edge is less than the width at the wall. The eccentricity of the vertical forces that cause the torsion is therefore less, while the section of the tread resisting the torsion is greater. In spiral stairs with rebated treads the horizontal forces developed by the rebates can be contrasted throughout the staircase by the enclosing wall, and it is likely that very little, if any, torsion is developed in the treads.

Failures and repairs

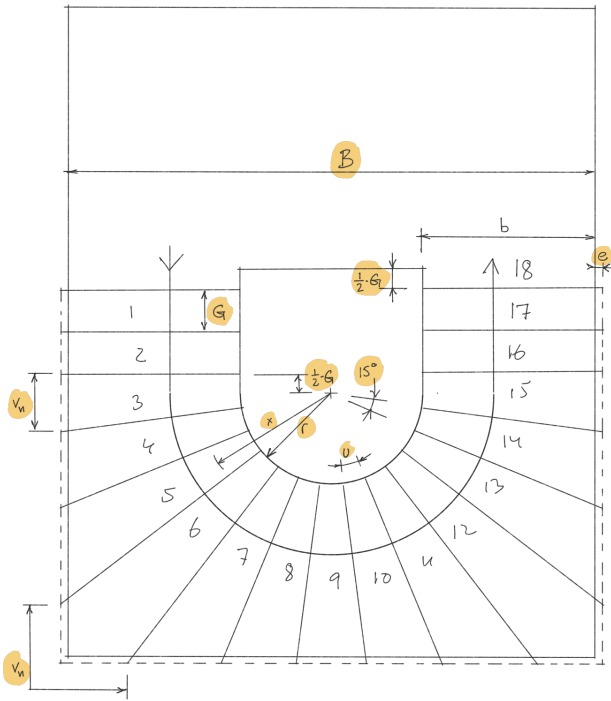
With such a large number of these stairs in use it is not surprising that there have been some failures. Like any piece of structure they can be overloaded, and unlike steelwork made out of



Trappeskitser



Trappeskitser



$$S = \frac{2924}{18} = 162,4444$$

$$B = 3335 \text{ mm}$$

$$G = 290 \text{ mm}$$

$$b = 1100 \text{ mm}$$

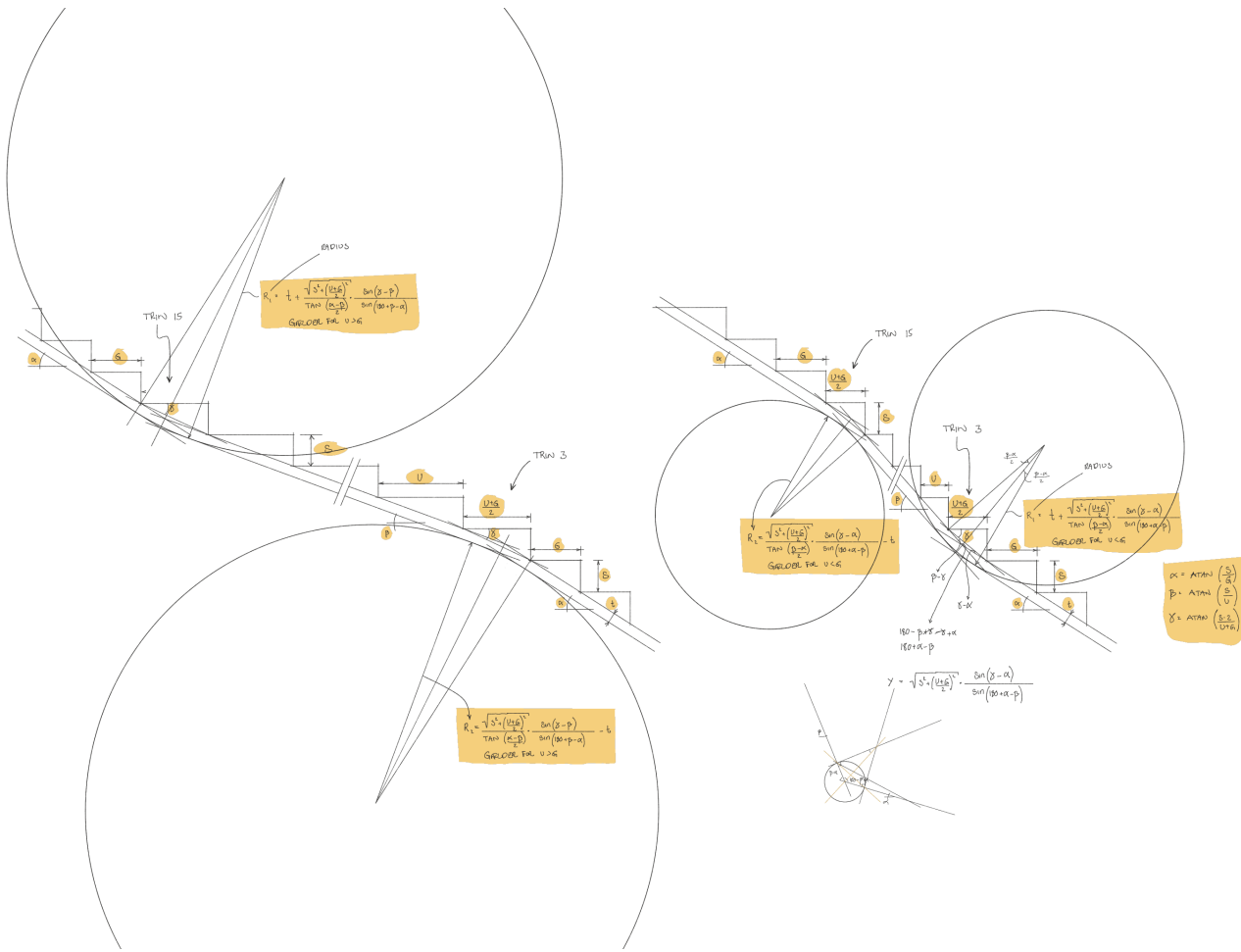
$$e = 30 \text{ mm}$$

$$r = \frac{B}{2} - b$$

$$\begin{aligned} V_2 &= V_{15} = \tan\left(\frac{15^\circ}{2}\right) \cdot (r+b+e) + \frac{G}{2} \\ V_4 &= V_8 = V_{10} = V_{14} = \left(\tan\left(15+15\frac{15^\circ}{2}\right) - \tan\left(\frac{15^\circ}{2}\right)\right) \cdot (r+b+e) \\ V_5 &= V_7 = V_{11} = V_{13} = \left(\tan\left(30+15\frac{15^\circ}{2}\right) - \tan\left(15+15\frac{15^\circ}{2}\right)\right) \cdot (r+b+e) \\ V_6 &= V_{12} = \left(\tan(45) - \tan\left(30+15\frac{15^\circ}{2}\right)\right) \cdot (r+b+e) \cdot 2 \\ V_9 &= \tan\left(\frac{15^\circ}{2}\right) \cdot (r+b+e) \cdot 2 \end{aligned}$$

$$U = \frac{\pi \cdot r \cdot 15^\circ}{180^\circ}$$

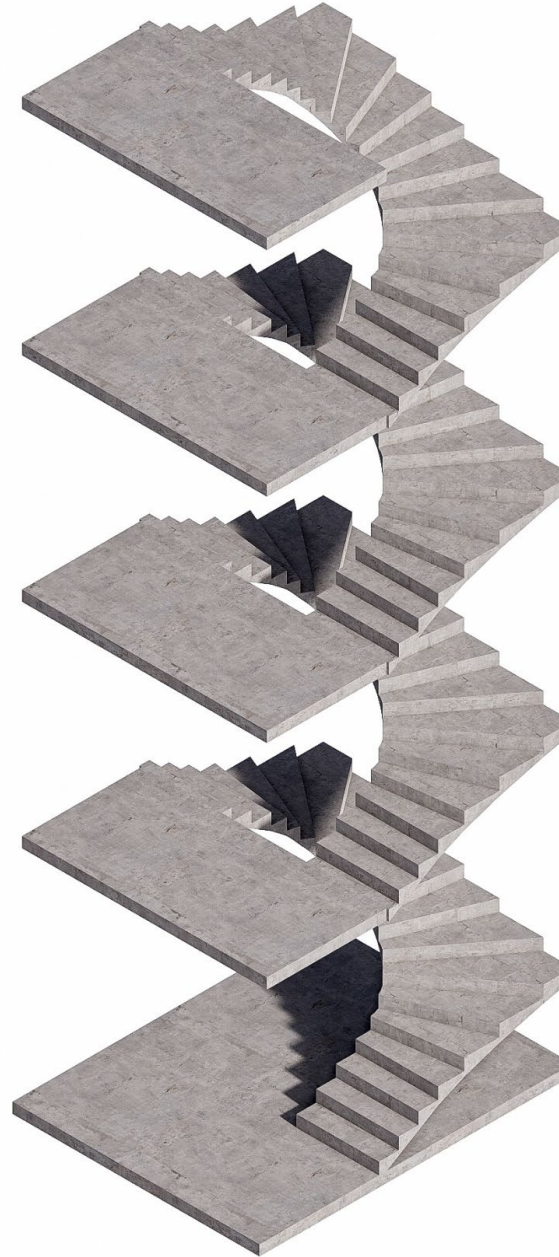
$$U(x) = \frac{\pi \cdot x \cdot 15^\circ}{180^\circ}$$



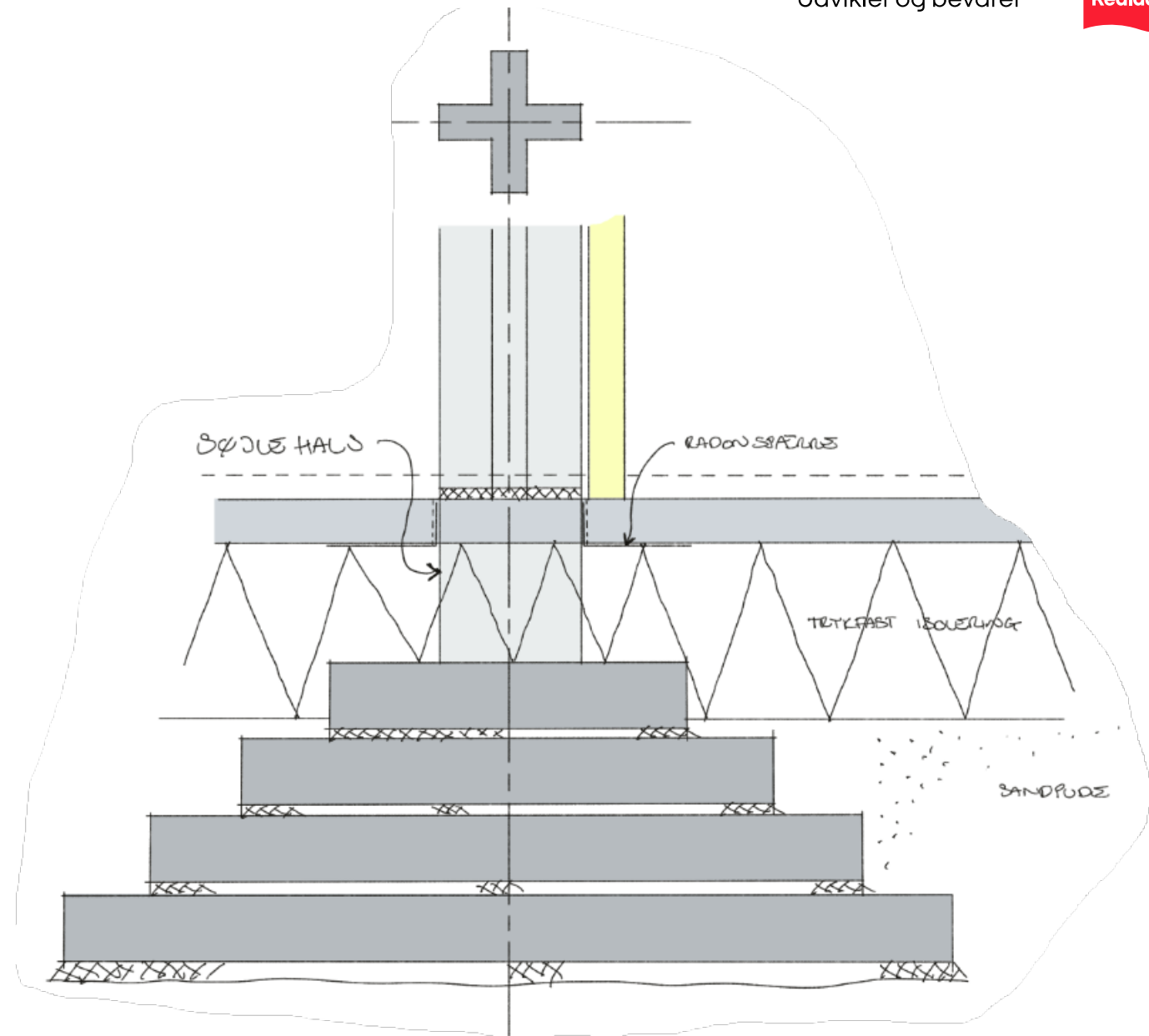
KONSTRUKTIONER

70% CO2 reduktion
sammenlignet med
standart
elementtrappe

Udvikler og bevarer

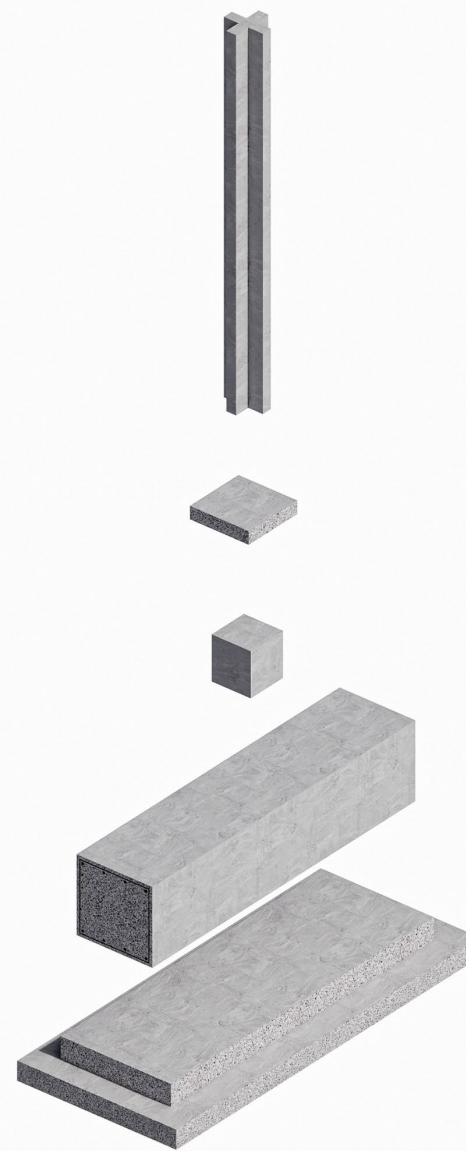
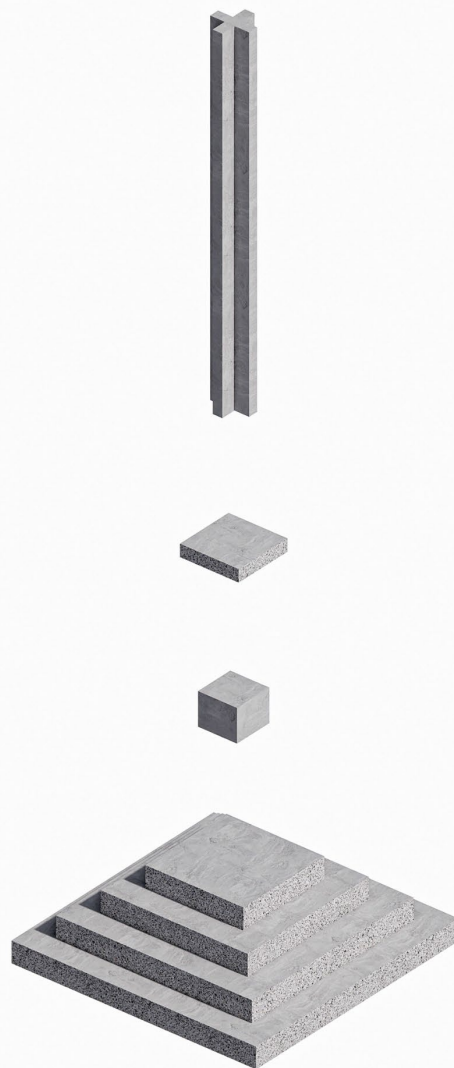


Fundamenter af genbrugte vægelementer



KONSTRUKTIONER

Fundamenter af genbrugte vægelementer



DOBBELT BUNDLINJE KONSTRUKTIONER **ARKITEKTUR**



ARKITEKTUR

Selvbærende, opmurede
facader

Enkle bærelinjer

Biogen isolering



Realdania
By & Byg

ARKITEKTUR

Spolia



ARKITEKTUR

**Betonsten af
genbrugselementer i
samarbejde med**

KINGO

SLAATTO MORSBØL

**BYGGESELSKABET CF
HANSEN**



ARKITEKTUR

Vægelementer Gellerupparken



Høst af elementer November 2024



ARKITEKTUR

Spolia



Realdania

Realdania
By & Byg

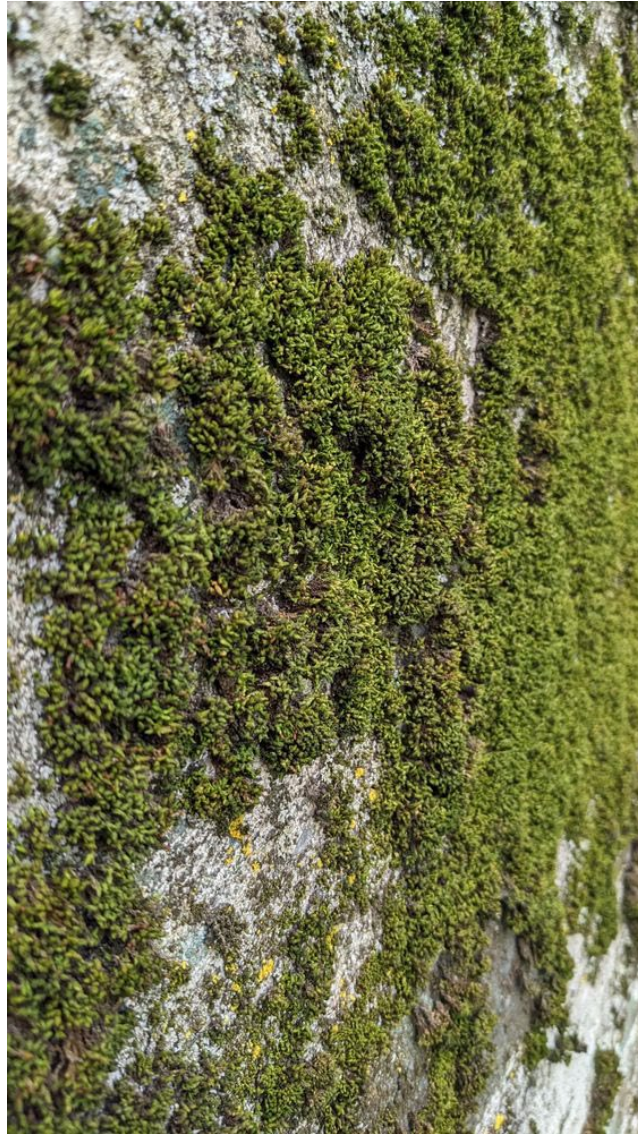
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Realdania

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ARKITEKTUR

Begrønning



ARKITEKTUR

Porebeton med
diffusionsåben puds

Optimerede
vinduesstørrelser

Passiv solaafskærmning

Passiv ventilation

Udnyttelse af termisk
masse





ARKITEKTUR

Indvendige vægge i
pudset porebeton

Gulve i genbrugt
bøgeparket fra
Gellerup Parken

Døre og inventar i
træ



KOMMENDE PROCES

**Ansøgning om
byggetilladelse i
december**

**Dialogfase under
Udførelsesprojekt**

**Bo Michelsen valgt
som
hovedentreprenør**



**Realдания
By & Byg**

